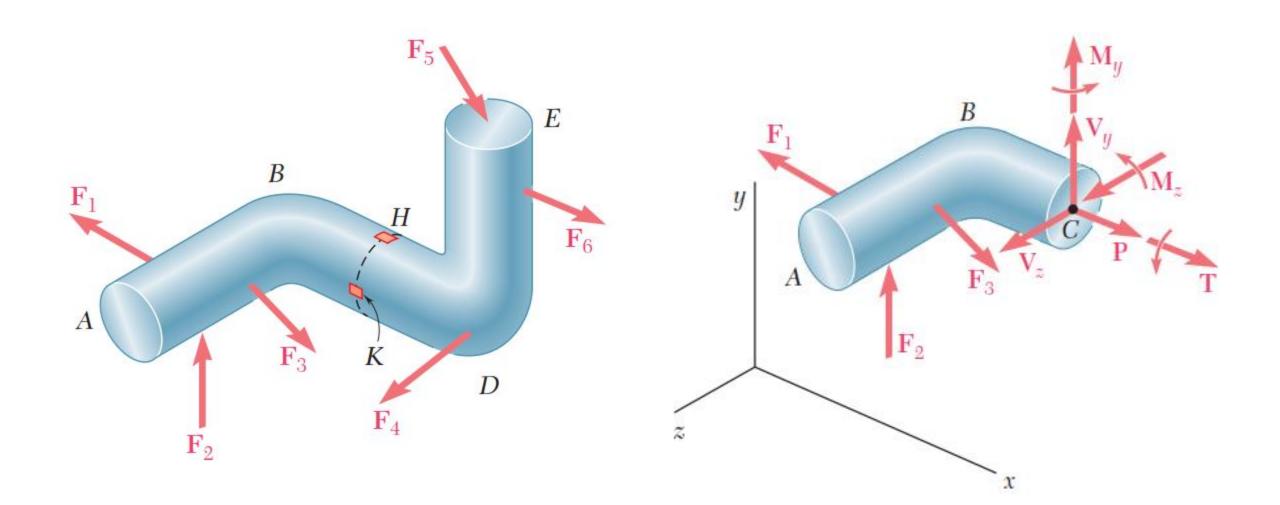




Objectives

 Determine stresses developed in a member's cross section when axial load, torsion, bending and shear occur simultaneously.

Stresses Under Combined Loads



Stresses Under Combined Loads

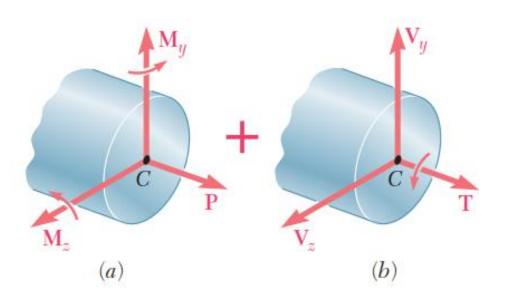


Fig. 8.17 Internal forces and couple vectors separated into (a) those causing normal stresses and (b) those causing shearing stresses.

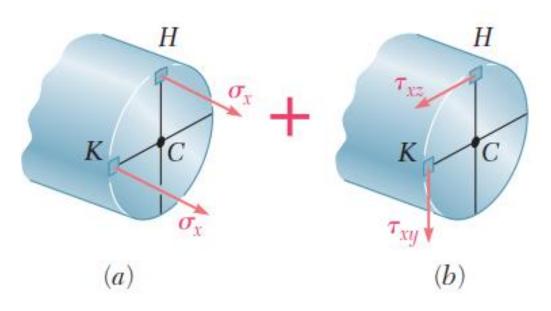


Fig. 8.18 Normal and shearing stresses at points H and K.

Stresses Under Combined Loads

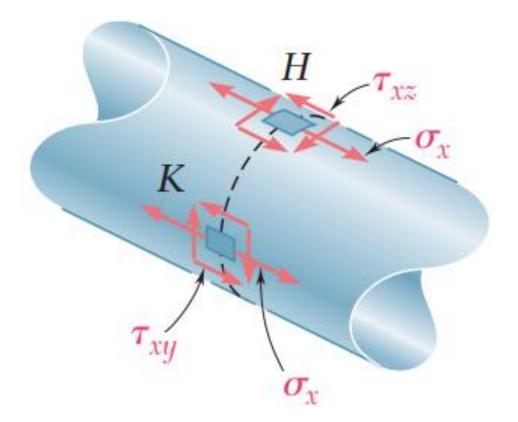
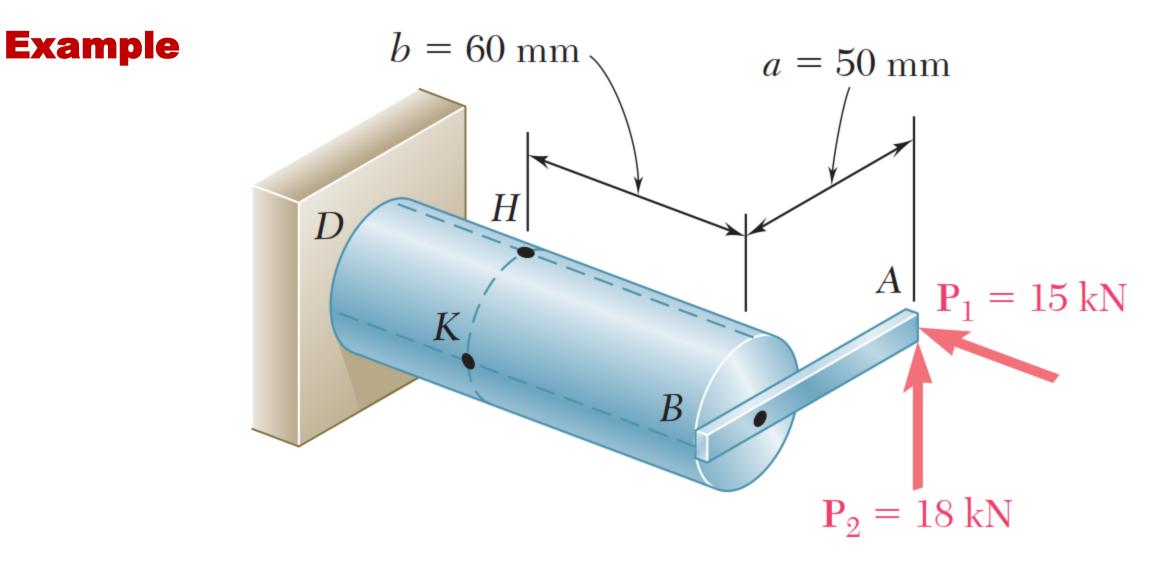
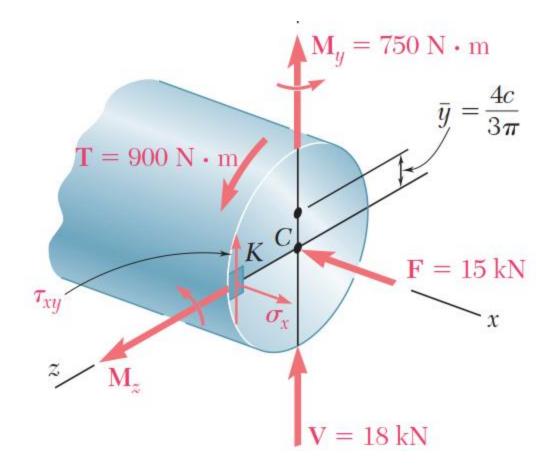


Fig. 8.19 Elements at points H and K showing combined stresses.



Determine (*a*) the normal and shearing stresses at point *K* of the transverse section of member *BD* located at a distance b = 60 mm from end *B*, (*b*) the principal axes and principal stresses at *K*, and (*c*) the maximum shearing stress at *K*.



 $F = P_1 = 15 \text{ kN}$ $V = P_2 = 18 \text{ kN}$ $T = P_2 a = (18 \text{ kN})(50 \text{ mm}) = 900 \text{ N} \cdot \text{m}$ $M_y = P_1 a = (15 \text{ kN})(50 \text{ mm}) = 750 \text{ N} \cdot \text{m}$ $M_z = P_2 b = (18 \text{ kN})(60 \text{ mm}) = 1080 \text{ N} \cdot \text{m}$

Geometric Properties of the Section

$$A = \pi c^{2} = \pi (0.020 \text{ m})^{2} = 1.257 \times 10^{-3} \text{ m}^{2}$$
$$I_{y} = I_{z} = \frac{1}{4} \pi c^{4} = \frac{1}{4} \pi (0.020 \text{ m})^{4} = 125.7 \times 10^{-9} \text{ m}^{4}$$
$$J_{C} = \frac{1}{2} \pi c^{4} = \frac{1}{2} \pi (0.020 \text{ m})^{4} = 251.3 \times 10^{-9} \text{ m}^{4}$$
$$Q = A' \bar{y} = \left(\frac{1}{2} \pi c^{2}\right) \left(\frac{4c}{3\pi}\right) = \frac{2}{3} c^{3} = \frac{2}{3} (0.020 \text{ m})^{3}$$

$$\begin{aligned} & = A y - \left(\frac{-\pi c}{2} \right) \left(\frac{-\pi c}{3\pi}\right) - \frac{-\pi c}{3} c &= -\frac{-\pi c}{3} (0.020) \\ & = 5.33 \times 10^{-6} \,\mathrm{m}^3 \end{aligned}$$

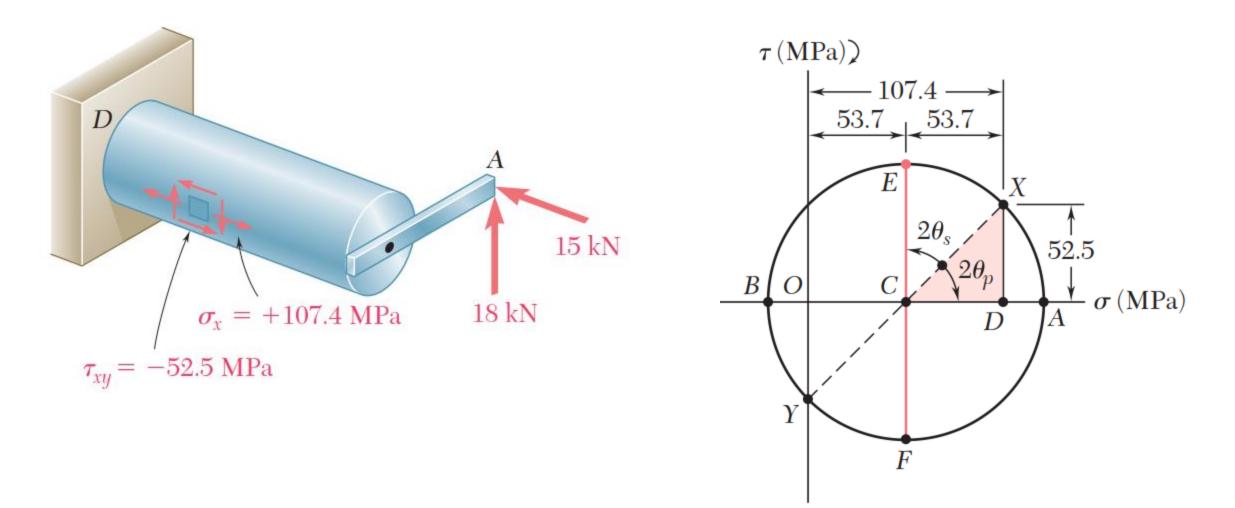
$$t = 2c = 2(0.020 \text{ m}) = 0.040 \text{ m}$$

 $\sigma_x = -\frac{F}{A} + \frac{M_y c}{I_y} = -11.9 \text{ MPa} + \frac{(750 \text{ N} \cdot \text{m})(0.020 \text{ m})}{125.7 \times 10^{-9} \text{ m}^4}$ = -11.9 MPa + 119.3 MPa $\sigma_x = +107.4 \, \text{MPa}$ $(\tau_{xy})_V = + \frac{VQ}{I_z t} = + \frac{(18 \times 10^3 \,\mathrm{N})(5.33 \times 10^{-6} \,\mathrm{m}^3)}{(125.7 \times 10^{-9} \,\mathrm{m}^4)(0.040 \,\mathrm{m})}$ = +19.1 MPa $(\tau_{xy})_{\text{twist}} = -\frac{Tc}{I_c} = -\frac{(900 \text{ N} \cdot \text{m})(0.020 \text{ m})}{251.3 \times 10^{-9} \text{ m}^4} = -71.6 \text{ MPa}$ $\tau_{xv} = (\tau_{xv})_V + (\tau_{xv})_{\text{twist}} = +19.1 \text{ MPa} - 71.6 \text{ MPa}$

Shearing Stresses.

Normal Stresses.

 $\tau_{xy} = -52.5 \text{ MPa}$

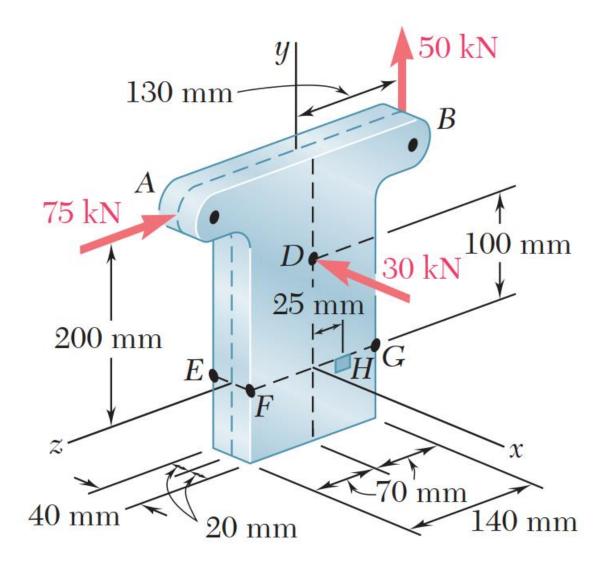


τ (MPa)) **Principal Planes and Principal Stresses at Point** *K*. 107.453.7 53.7 $\tan 2\theta_p = \frac{DX}{CD} = \frac{52.5}{53.7} = 0.97765$ $2\theta_p = 44.4^\circ$) $\theta_p = 22.2^\circ$ $2\theta_s$ 52.5 $R = \sqrt{(53.7)^2 + (52.5)^2} = 75.1 \text{ MPa}$ $2\theta_p$ B σ (MPa) D $\sigma_{\rm max} = OC + R = 53.7 + 75.1 = 128.8 \,{\rm MPa}$ $\sigma_{\min} = OC - R = 53.7 - 75.1 = -21.4 \text{ MPa}$ γ F

Maximum Shearing Stress at Point *K*.

$$\tau_{\rm max} = CE = R = 75.1 \,\mathrm{MPa} \qquad \theta_s = 22.8^\circ\,\mathrm{V}$$

Example 2



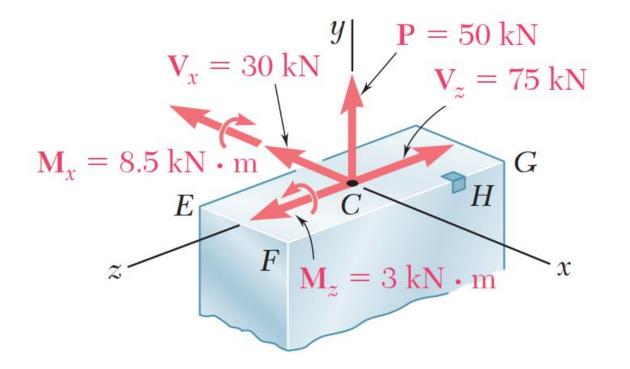
For the loading shown determine the principal stresses, principal planes, and maximum shearing stress at point *H*.

$$V_x = -30 \,\mathrm{kN} \qquad P = 50 \,\mathrm{kN} \qquad V_z = -75 \,\mathrm{kN}$$

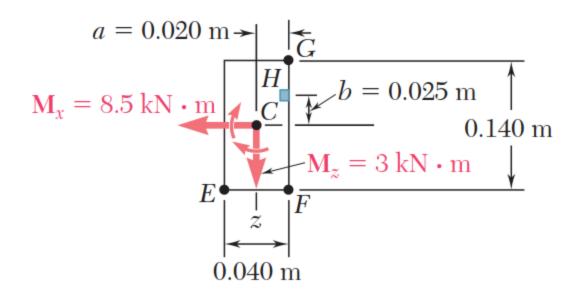
Internal Forces in Section *EFG*.

$$M_x = (50 \text{ kN})(0.130 \text{ m}) - (75 \text{ kN})(0.200 \text{ m}) = -8.5 \text{ kN} \cdot \text{m}$$

$$M_y = 0$$
 $M_z = (30 \text{ kN})(0.100 \text{ m}) = 3 \text{ kN} \cdot \text{m}$

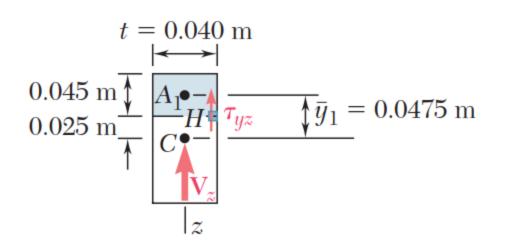


 $A = (0.040 \text{ m})(0.140 \text{ m}) = 5.6 \times 10^{-3} \text{ m}^2$ $I_x = \frac{1}{12}(0.040 \text{ m})(0.140 \text{ m})^3 = 9.15 \times 10^{-6} \text{ m}^4$ $I_z = \frac{1}{12}(0.140 \text{ m})(0.040 \text{ m})^3 = 0.747 \times 10^{-6} \text{ m}^4$



Normal Stress at *H*.

$$\sigma_{y} = +\frac{P}{A} + \frac{|M_{z}|a}{I_{z}} - \frac{|M_{x}|b}{I_{x}}$$
$$= \frac{50 \text{ kN}}{5.6 \times 10^{-3} \text{ m}^{2}} + \frac{(3 \text{ kN} \cdot \text{m})(0.020 \text{ m})}{0.747 \times 10^{-6} \text{ m}^{4}} - \frac{(8.5 \text{ kN} \cdot \text{m})(0.025 \text{ m})}{9.15 \times 10^{-6} \text{ m}^{4}}$$
$$\sigma_{y} = 8.93 \text{ MPa} + 80.3 \text{ MPa} - 23.2 \text{ MPa} \qquad \sigma_{y} = 66.0 \text{ MPa}$$



Considering the shearing force V_x , we note that Q = 0 with respect to the *z* axis, since *H* is on the edge of the cross section. Thus, V_x produces no shearing stress at *H*.

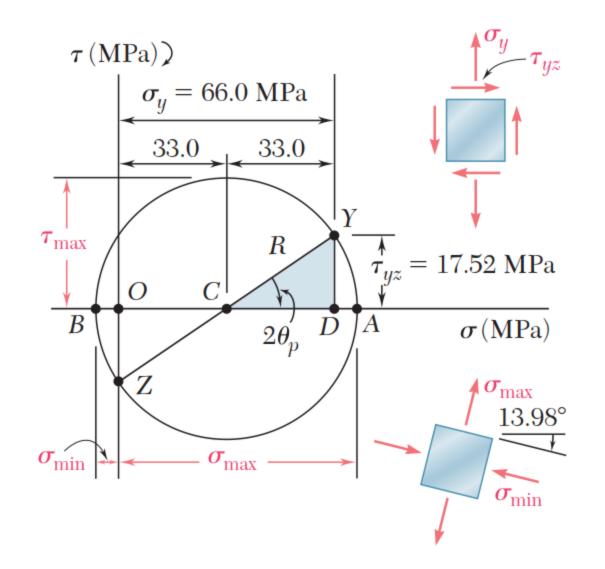
The shearing force V_z does produce a shearing stress at H

Shearing Stress at *H***.**

$$Q = A_1 \bar{y}_1 = [(0.040 \text{ m})(0.045 \text{ m})](0.0475 \text{ m}) = 85.5 \times 10^{-6} \text{ m}^3$$

$$\tau_{yz} = \frac{V_z Q}{I_x t} = \frac{(75 \text{ kN})(85.5 \times 10^{-6} \text{ m}^3)}{(9.15 \times 10^{-6} \text{ m}^4)(0.040 \text{ m})} \qquad \tau_{yz} = 17.52 \text{ MPa}$$

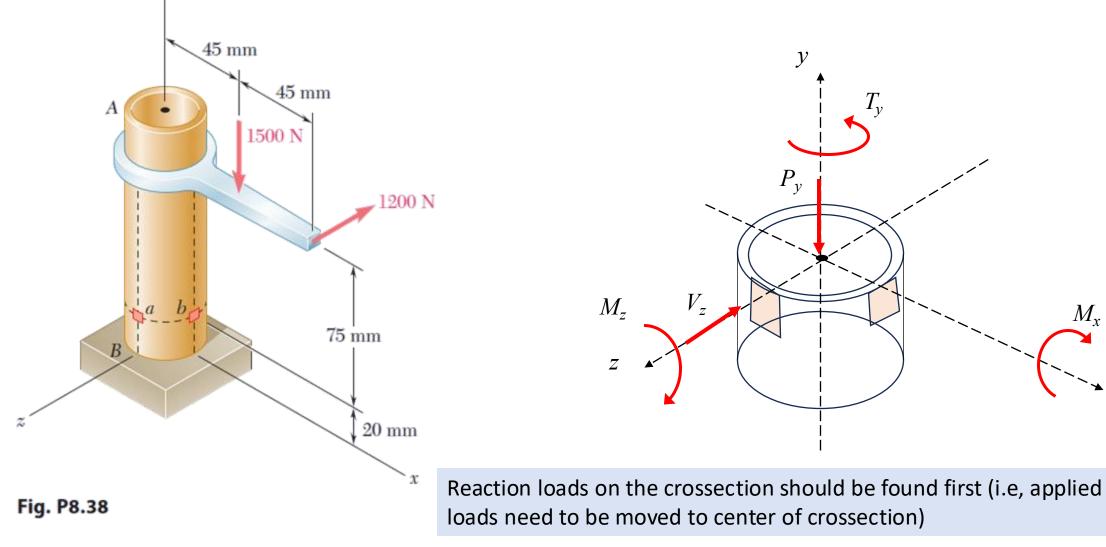
Principal Stresses, Principal Planes, and Maximum Shearing Stress at *H***.**

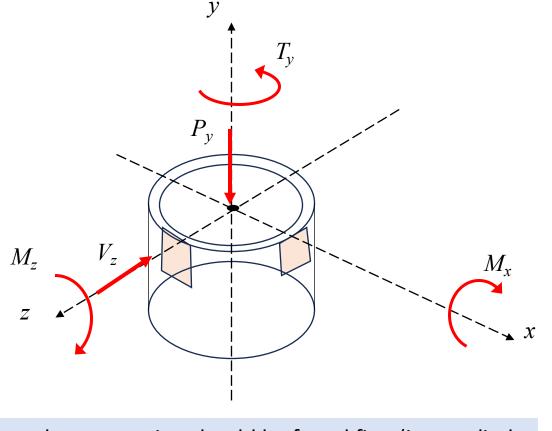


Problem 8.38

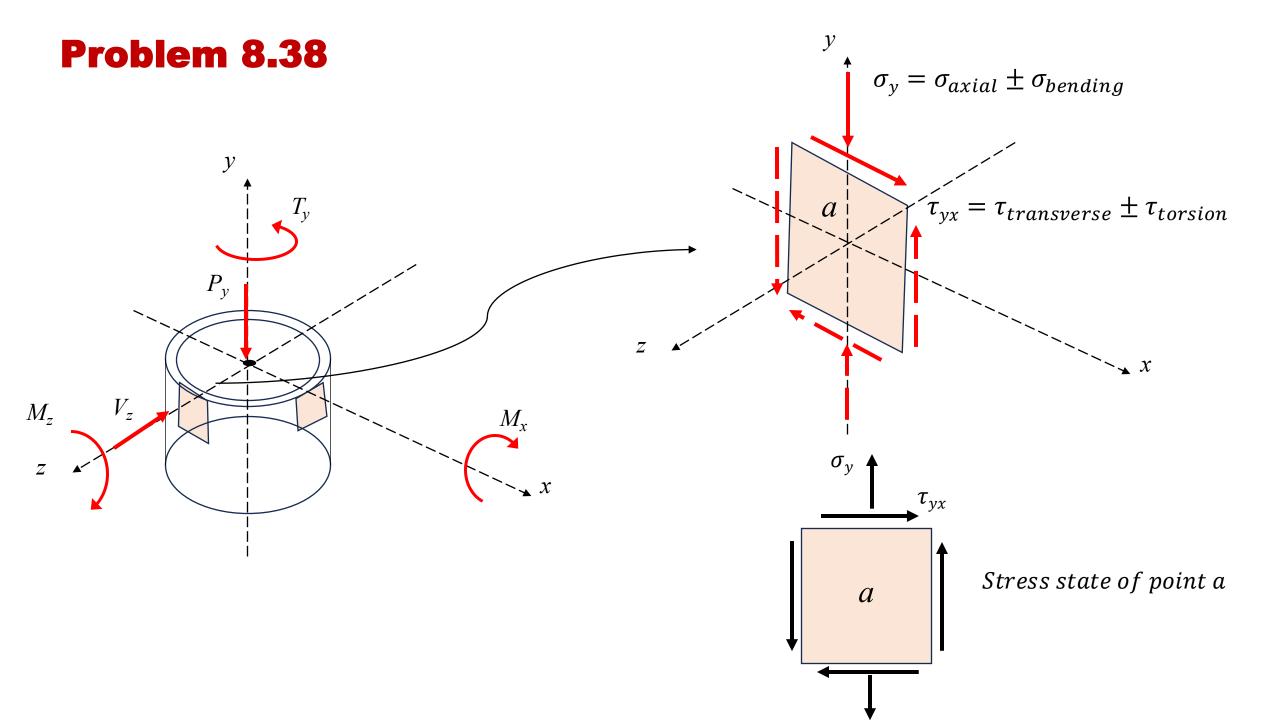
y

Two forces are applied to the pipe AB as shown. Knowing that the pipe has inner and outer diameters equal to 35 and 42 mm, respectively, determine the normal and shearing stresses at (a) point a, (b) point b.





(a) 20.4 MPa; 14.34 MPa. (b) -21.5 MPa; 19.98 MPa.



Problem 8.53

Three steel plates, each 13 mm thick, are welded together to form a cantilever beam. For the loading shown, determine the normal and shearing stresses at points a and b.

